

1. Determine the Design Wind Load

[1.1] Using the Analytical Method ASCE 7 05

The procedure to determine Design Wind Load is specified by the American Society of Civil Engineers (ASCE) and referenced in the International Building Code (IBC) 2006. For the purposes of this document, the values, equations, and procedures used in this document reference ASCE 7 05, Minimum Design Loads for Buildings and Other Structures. Please refer to ASCE 7 05 if you have any questions about the definitions or procedures presented in this manual.

The wind force analysis is based on ASCE 7 05, Chapter 6. Upon review of the different methods available for calculating wind loading, it has been determined that Method 2 (Analytical Procedure) is most appropriate. Specifically Section 6.5.13 (Design Wind Loads on Open Buildings with Mono sloped, pitched, or Troughed Roofs), was selected as the method most closely approaching the application of Solar Matrix racking system. The pressures are determined following Section 6.5.13.2 (Main Wind Force Resisting System) according to the following formula:

$$p = qh \times C_n \quad \text{Equation 1}$$

Where

p = design wind pressure (+ denotes down towards the roof)

qh = velocity pressure evaluated at mean roof height

G = gust effect factor as determined in ASCE 7 05 Section 6.5.8

C_n = net pressure coefficient determined from ASCE 7 05 Fig. 6.18A, p.66.

[1.2] Determine the Velocity Pressure according 6.5.10

$$q_z = q_h = 0.00256 K_z \times K_{zt} K_d \times V^2 = 17.63 (\text{lb/ft}^2, \text{psf})$$

Wind Speed, $V = 100 \text{ mph} (45 \text{ m/s})$

Importance Factor $I = 1$ (Table 6-1)

Direct. Factor, $K_d = 0.85$ (Table 6-4)

Topo. Factor, $K_{zt} = 1.00$ (Sect. 6.5.7)

Topographic Factor, $K_z = 0.81$ (Table 6-3)

[1.3] Determine the Gust Effect Factor, G

The gust factor, G, is determined to equal 0.85, according to Equation 1, then going to ASCE 7 05 Section 6.5.8.1, and using the structure definition from ASCE 7 05, Section 6 2.

[1.4] Determine the Net Pressure Coefficient, Cn

The net pressure coefficient, Cn, can be determined from ASCE 7 05 Fig. 6 18A, p.66.

Cn=-0.9

Cn=-1.3

[1.5] Calculate the Design Wind Load, p (psf)

The max uplift Velocity Pressure = $q_h \times G \times C_n = -13.49 \text{ psf}$

The max down Velocity Pressure = $q_h \times G \times C_n = 19.48 \text{ psf}$

2. Load Forces on Solar Matrix Mounting System

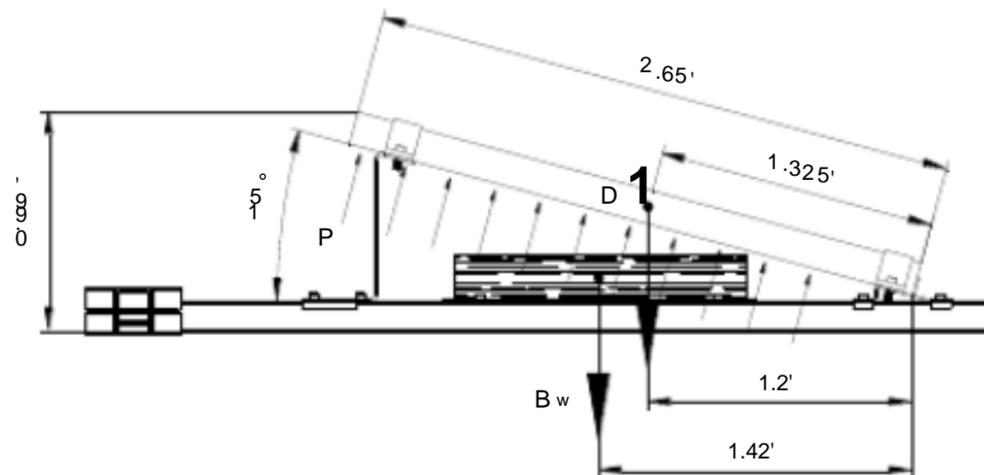
The solar panel size is 5.18' x 2.65' x 0.15'

The weight is 18 Kg.

So the module area = 5.18' x 2.65' = 13.73 sq ft

The Dead load (not include the ballast) $D_1 = 59 \text{ lbf}$

The total occupation area for the simplify system is A about 19.67 sq ft



OVER VIEW FOR THE SOLAR MATRIX STRUCTURE

For a large Solar Matrix system, the danger is to be turnover by the uplift wind force. We can check the panels on the last row to ensure the system safety.

The force on the panels on the last row must keep equal. From the figure, we know:

$$B_w \times 1.42 + D_1 \times 1.2 = W_{lu} \times 1.325$$

$$W_{lu} = \text{max uplift Velocity Pressure} \times \text{the module area} = 19.48 \times 13.73 = 267.5 \text{ lbf}$$

So,

$$B_w = (W_{lu} \times 1.325 - D_1 \times 1.2) / 1.42 = (267.5 \times 1.325 - 59 \times 1.2) / 1.42 = 199.7 \text{ lbf}$$

The safety factor is 1.5,

So,

$$B_w = 1.5 \times 199.7 = 299.6 \text{ lbf (It means each ballast weight is 135 kg)}$$

From the above, the total dead load to roof is:

$$D = D_1 + B_w = 299.6 + 59 = 358.6 \text{ lbf}$$

So the uniformly distributed load for the Solar Matrix is :

$$q = D/A = 358.6/19.67 = 18.23 \text{ psf}$$